



1st International Workshop on Asphalt Recycling Technologies  
9-10 September, 2024 – RWTH Aachen, Germany

# The Italian Experience on Warm-Recycled Asphalt Mixes: from Research to Motorway Construction Specifications

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**DICEA** UNIVPM

DIPARTIMENTO INGEGNERIA  
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18 | 22 23 | 27 ECCELLENZA



1. Introduction
2. Trial Section 2016
3. Conventional mechanical testing
4. S-VECD performance analysis
5. Interlayer shear bonding
6. FWD monitoring
7. Chemical evaluation of recovered PmB
8. Trial Section 2022
9. Environmental assessment
10. Five-step classification of WMA additives
11. Construction Specifications & Conclusions





Highway Pavement Evolutive Research

Guidance and Coordination Unit:



Academic Research Unit:



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DELLE MARCHE

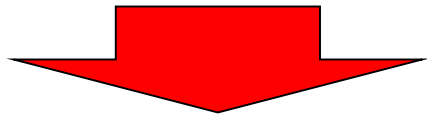
Operational Support Units:



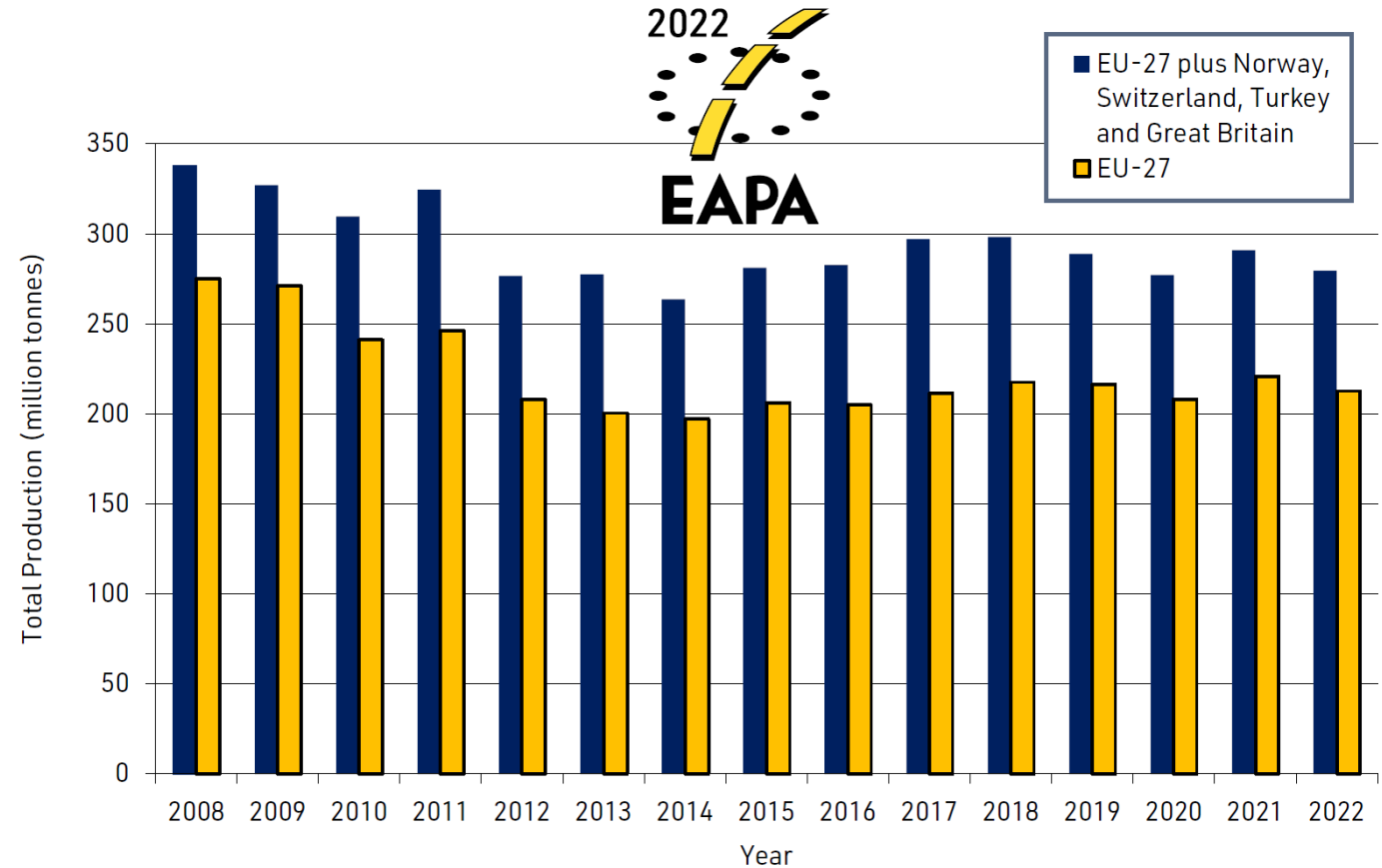
## Reclaimed Asphalt Pavement Surplus

Total EU-27HMA and WMA production 2022

# 279,4 Mtonnes



- Mainly maintenance work
- HWA and WMA production = RAP
- Average RAP re-use = 30%
- **Huge amount of RAP surplus**



Source: Asphalt In Figures 2022 - EAPA



## RAP as New Construction Material (no recycled material anymore)

Reclaimed Asphalt Pavement

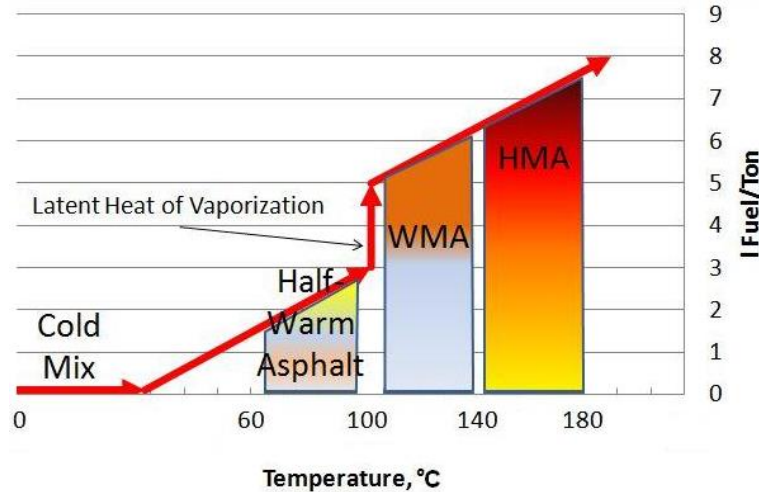


**Hot and Warm Recycled Asphalt mixes:**  
Increase from current max 30%  
to a viable **40-50% re-use**

**Cold Recycled Mixtures:**  
Up to **90-100% re-use**

- Motorway maintenance work → mainly AC surface layers (no cold recycling)
- Cold Recycling → RAP = Black Rock (no reactivation)
- Bitumen → material from non-renewable resource:  
limited availability and negative outlook

# Possible Solution for Sustainable Paving Mixtures: WMA Technology



- Reduction of production temperatures & energy savings
- Lower pollutant emissions & improved working conditions
- Longer hauling distances, extended time for paving, faster opening to traffic
- Lower bitumen aging & possibility to add higher %RAP

## Limited data concerning:

- WMA long-term field performance
- Interaction between WMA additives and PmB

## Autostrade per l'Italia Motorway Network ( $\approx 3000$ km)



- ❑ The largest Highway Operator in Europe
- ❑ High heavy-traffic volume → use of **SBS PmB**
- ❑ Continuous rehabilitation and maintenance activities
- ❑ Excess of huge amount of RAP ( $\approx 70\%$ )
- ❑ Narrow territory & Densely populated
- ❑ **Plants located near urban areas**

Challenge:

How can we **reduce emissions** and **incorporate higher RAP** amount **without penalizing performance**?

## The Beginning



12 Asphalt Batch Mix Plant  
(some of them near urban areas)



**HMA with PmB**

**WMA with PmB**

**STANDARD PRODUCTION T**

**LOW PRODUCTION T**

MIXING = 170 °C

MIXING = 130 °C

COMPACTION = 160 °C

COMPACTION = 120 °C

**- 40 °C**





## Which Additive?

Organic additive

O



To lower the viscosity at  $T > 90\text{ }^{\circ}\text{C}$ .  
The additive is a special paraffin wax with long hydrocarbon chains.

Zeolite (water containing) Z



A foaming technique uses hydrophilic minerals with 20% of crystalline water, released at  $T > 100\text{ }^{\circ}\text{C}$ .

Chemical additive

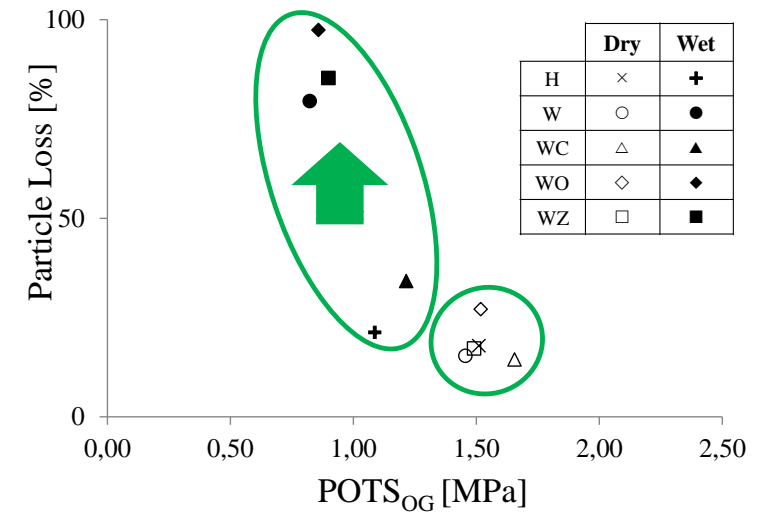
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Do not change the bitumen viscosity. They act as surfactants at the binder interface by reducing frictional forces

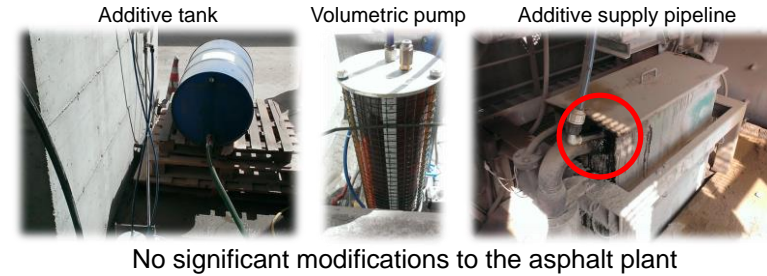
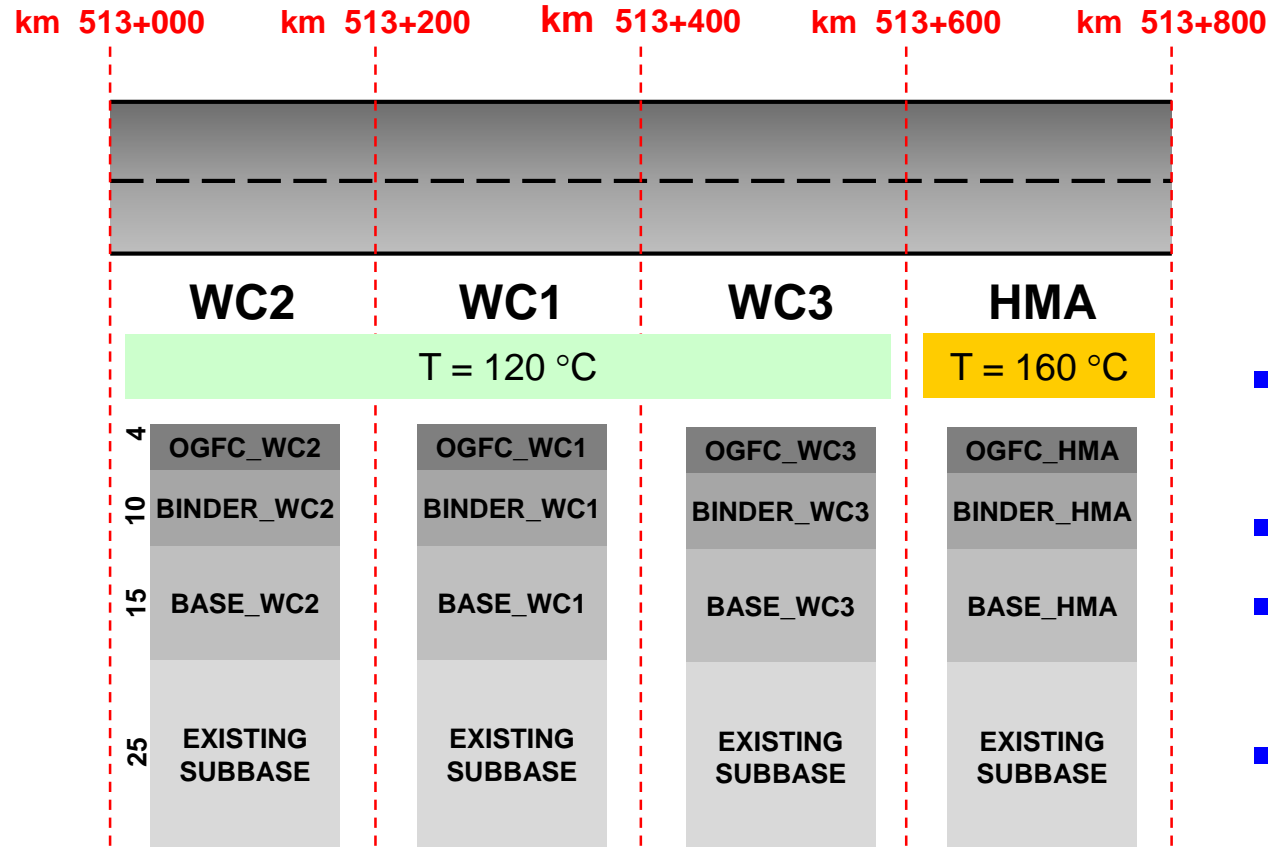
**Wet conditions:** only warm OG mixtures with Chemical Additive (WC) show good raveling performance

**BASALT / OG MIXTURES**



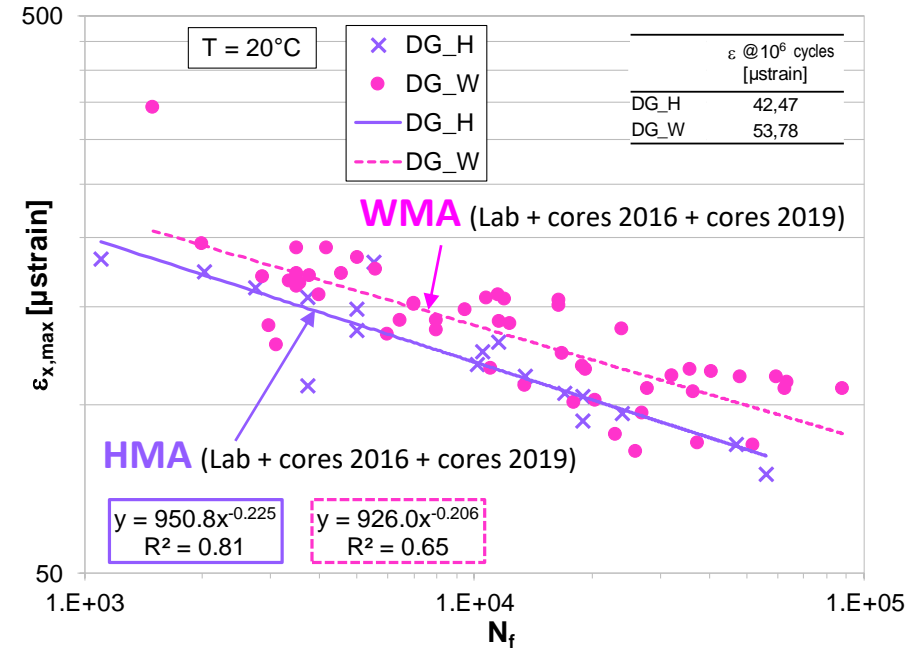
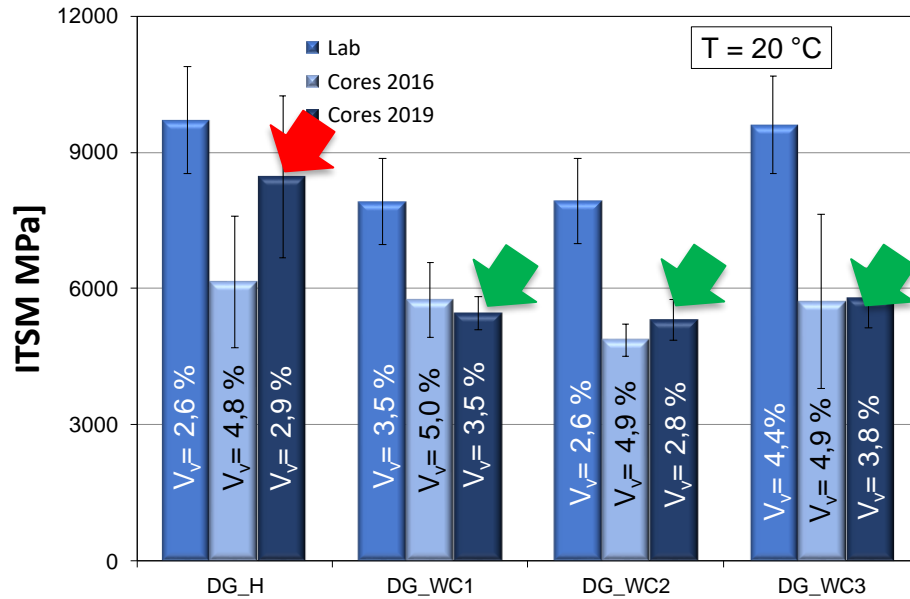
Selected WMA Technology for both DG and OG mixes

# Motorway A1: 4 sections all containing PmB



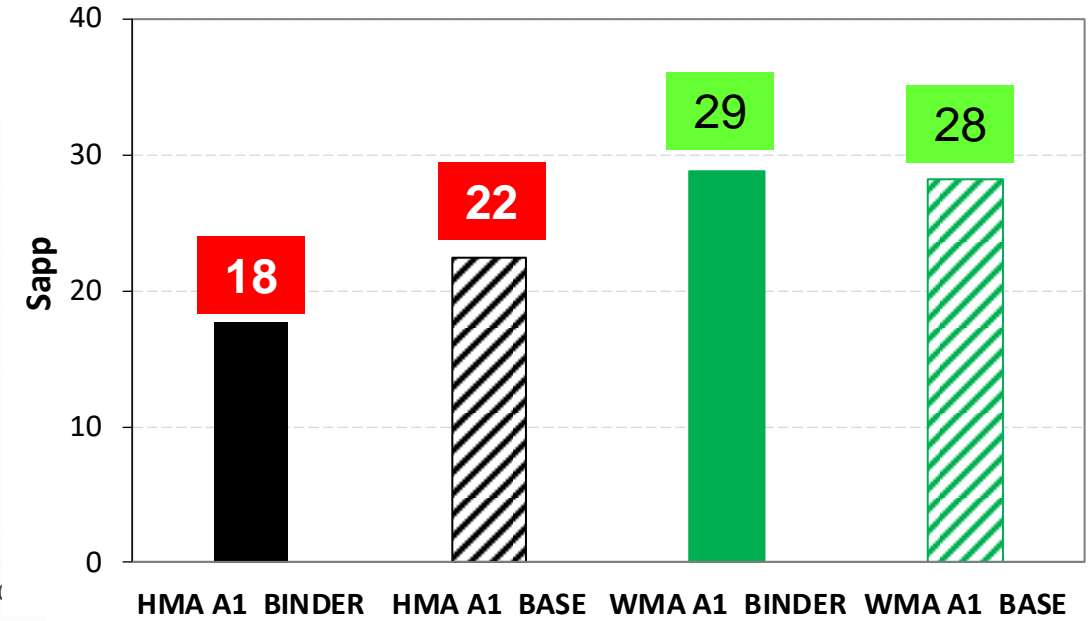
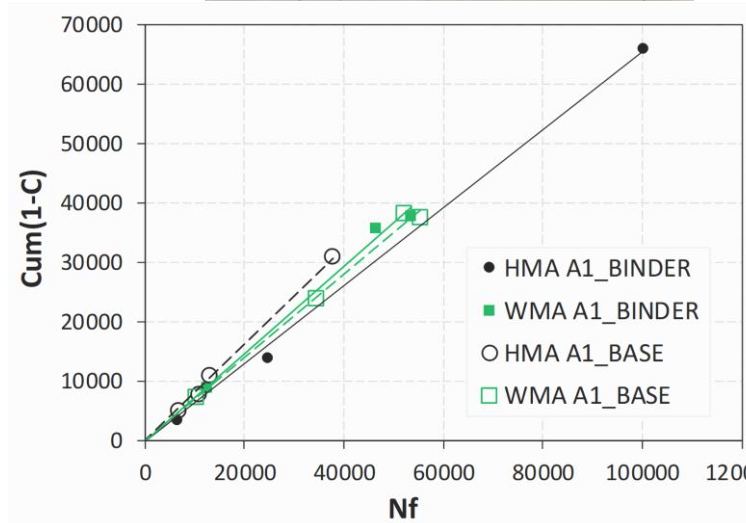
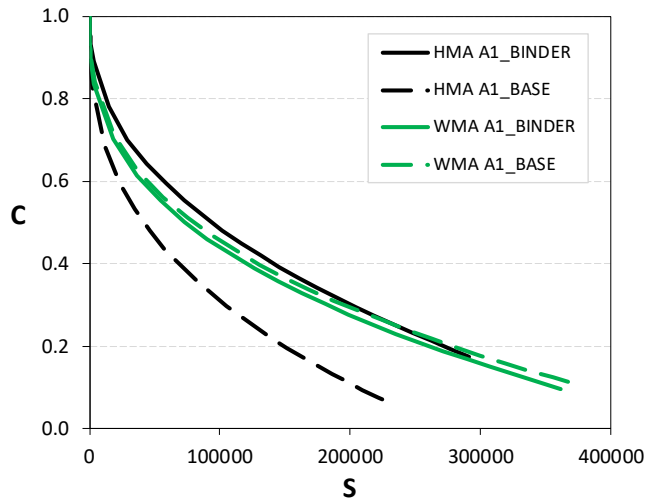
- **Full-depth repair** after milling all existing asphalt layers
- 1 reference section with **hot recycled** mixes
- 3 sections: **warm recycled** mixtures with different chemical additives C1, C2, C3
- Same **climatic conditions** and **annual traffic** (**8,5 million ESALs 120 kN**)

## ITSM & ITF tests on laboratory specimens and in-situ cores (2016, 2019)



- Lab and cores 2016: similar ITSM values for **warm** and **hot** mixtures
- Cores 2019: higher variability for HMA and noticeable ITSM increase (severe aging effect?)
- Resistance to cyclic loading: **WMA > HMA**

## HMA vs WMA\_C1 in-situ cores (2022)



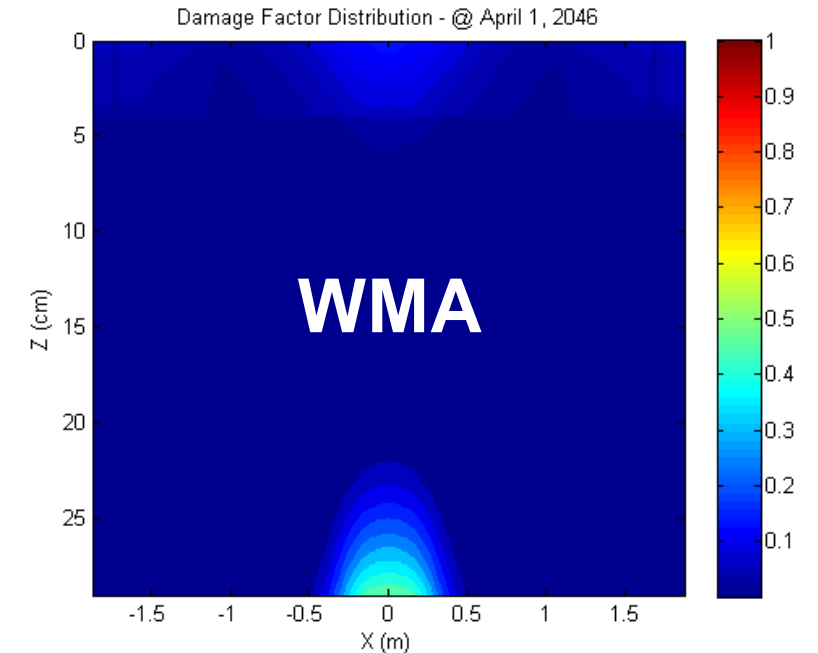
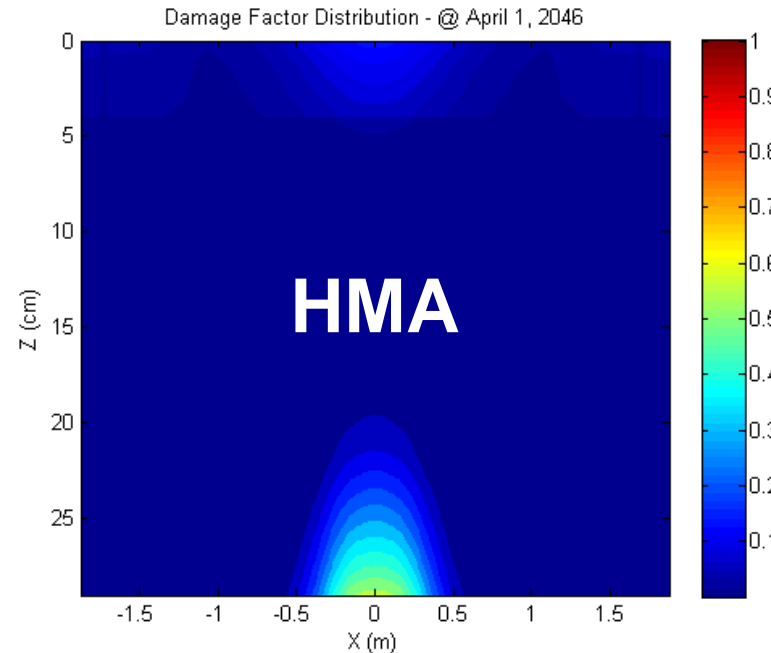
■ Sapp values → based on the *C-S curves* and  $D^R$  values

→ **A better fatigue performance can be expected from WMA,**  
similar for both binder and base layers, thanks to the **lower RAP oxidation**

### HMA vs WMA\_C1 in-situ cores (2022)

#### Damage contours

- 25 cm subbase  $E = 1200$  MPa
- Prediction after 30 years
- Fatigue + climatic damage
- 8.5 mln – 120 kN ESALs
- $v = 90$  km/h



- Limited **top-down cracking** due to **thermal effects** in both sections
- **Bottom-up cracking** → about **9 cm** in **HMA** section vs. about **6 cm** in **WMA** section
- **Higher performance of WMA mixtures**

## Background

Road Pavement = Multilayered structure



Overall performance depends on  
**Interlayer bonding behavior**

Full  
Adhesion

Perfectly  
Smooth



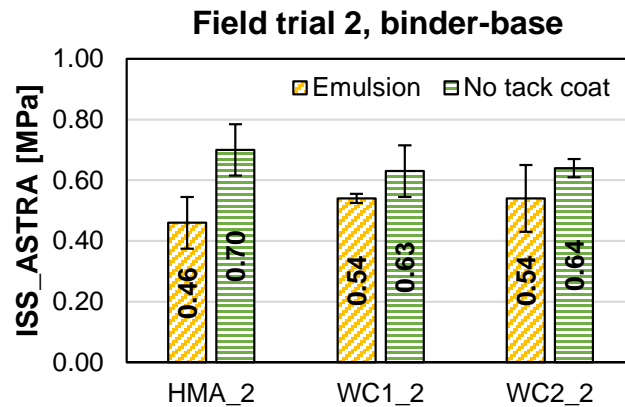
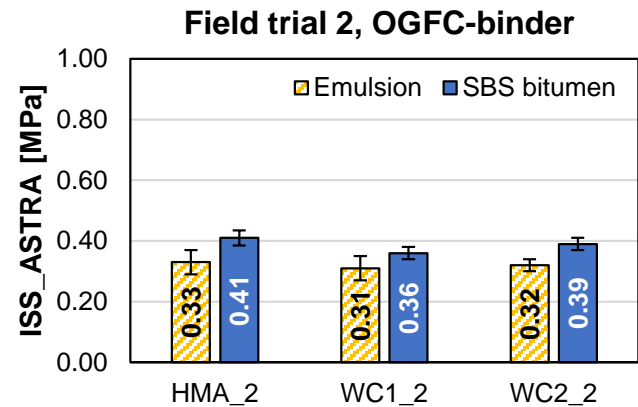
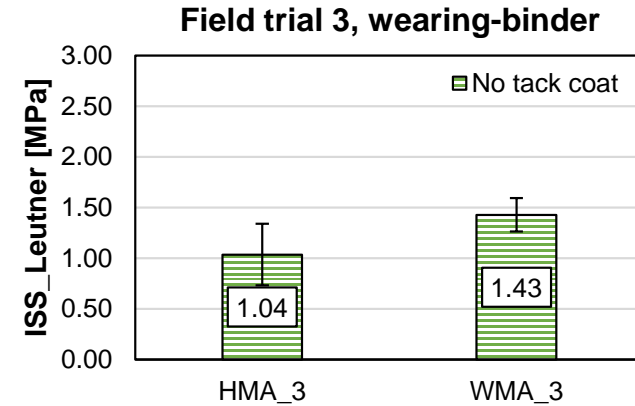
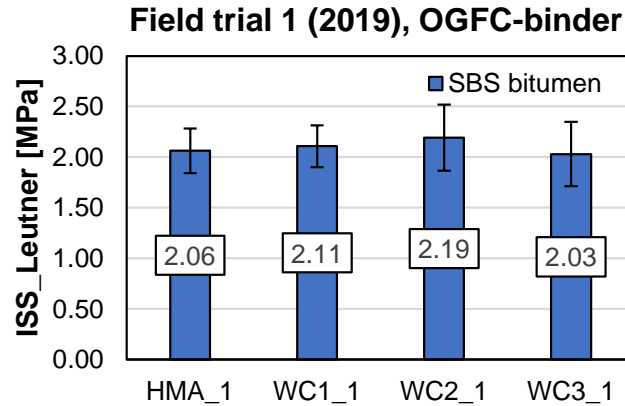
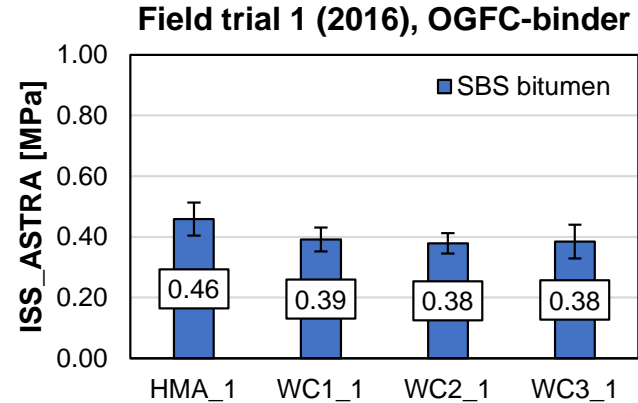
**Real behavior: Intermediate & Evolutive**



## Effects of WMA technology

### Variables investigated

- Effect of WMA technology
- Effect of WMA additive type
- Effect of age
- Effect of tack coat properties
- Effect of mixture type
- Influence of asphalt plant

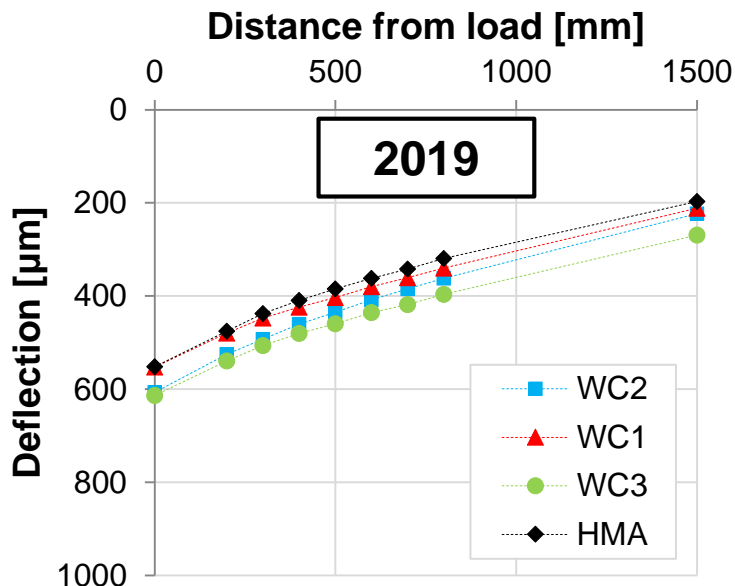
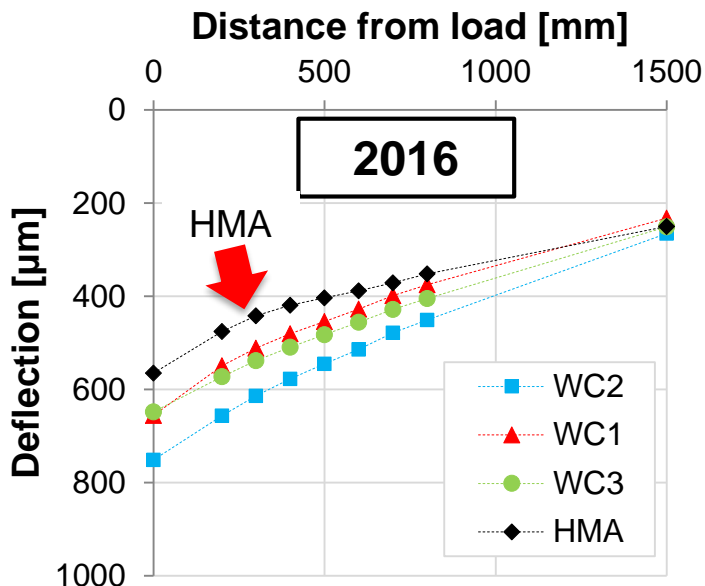


One-factor analysis of variance (ANOVA) with confidence level of 95%

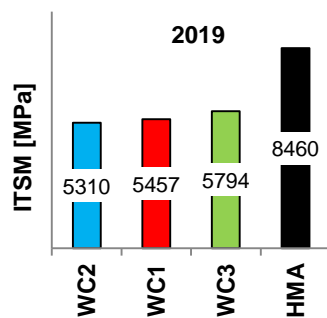
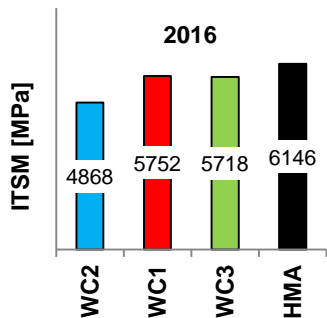
in most cases the **differences** between HMA and WMA are not statistically significant

- The reduced lay-down temperature of warm mixtures does not penalize the interlayer bonding**

## Representative FWD deflection basins – 2016 and 2019



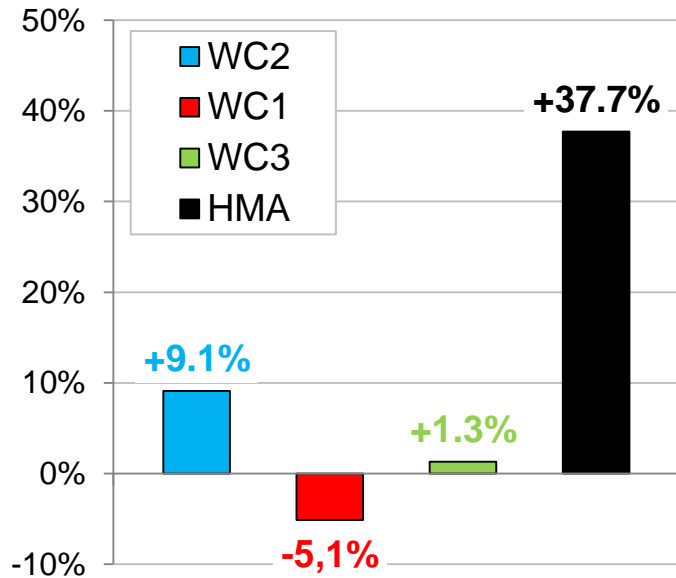
- ☐ 2016: good correspondence between deflections and ITSM
- ☐ 2016 and 2019: limited influence of the lower layers
- ☐ 2019 vs. 2016: changes attributable to the asphalt layers



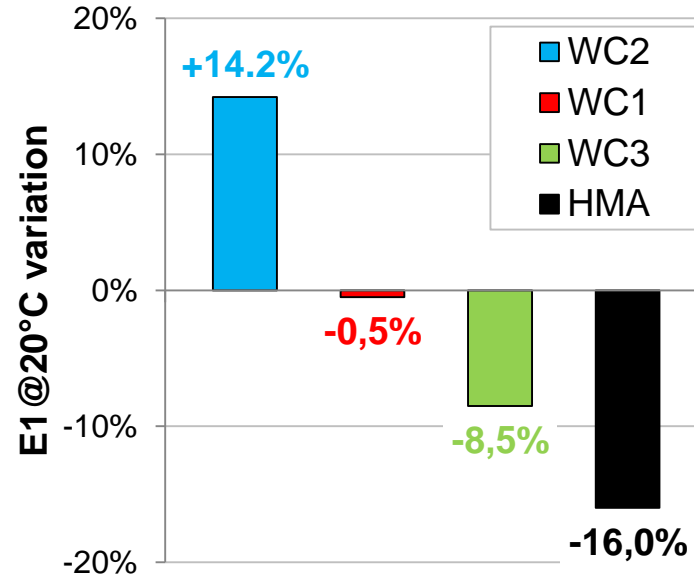


## Interpretation of the results

ITSM variation (2019 vs. 2016)



E1@20 °C variation (2019 vs. 2016)



- ☐ WC2: increase ITSM and E1@20 °C
- ☐ WC1 and WC3: almost no variation
- ☐ **HMA: increase ITSM and decrease in E1@20 °C**



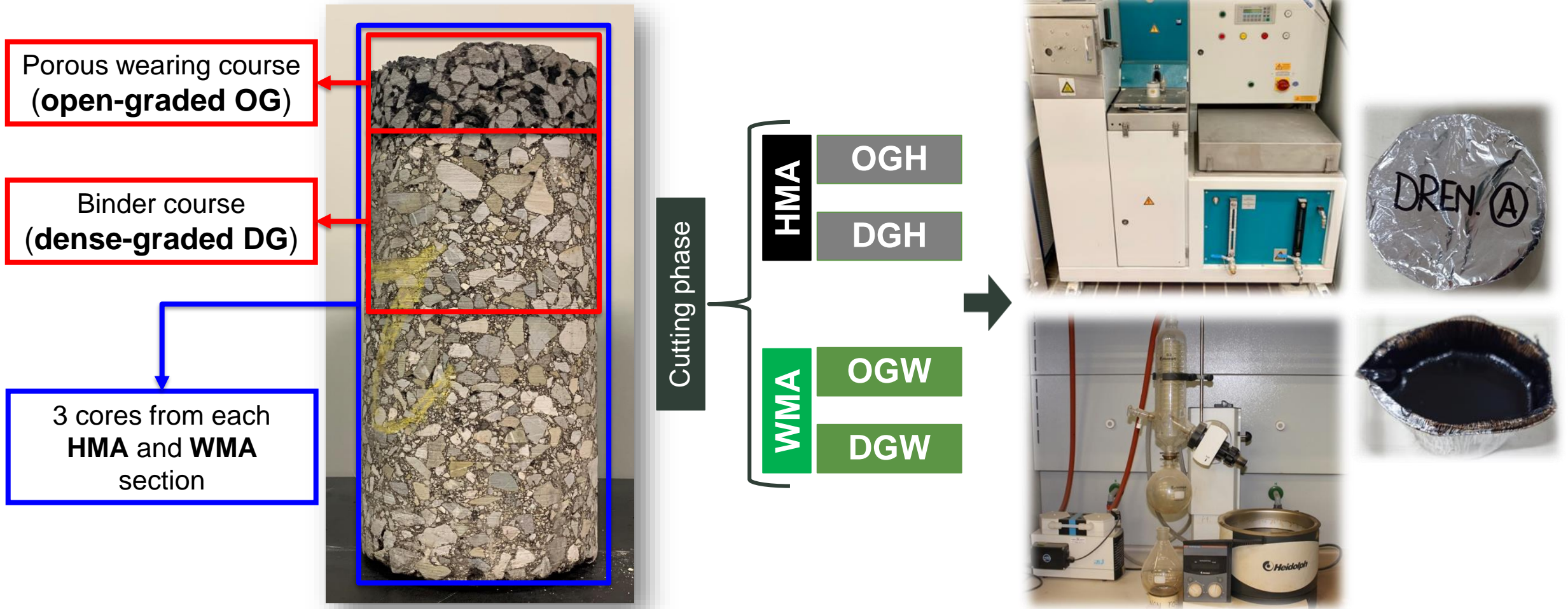
The back-calculation expresses the layer performance, which corresponds to the mixture performance only if the layer is intact



**HMA experienced the most severe aging → greater fracture and damage tendency**

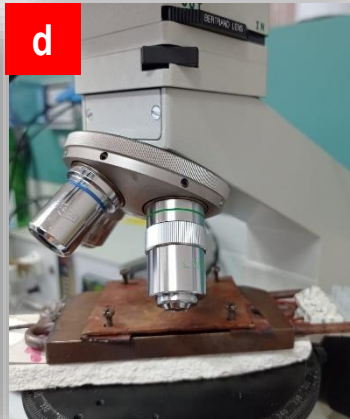
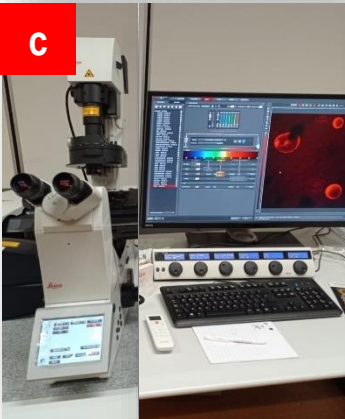
**Warm recycled mixes may ensure higher performance and longer service life than hot recycled mixes**

## Recovered PmB: HMA & WMA\_C1 in-situ cores (2022)

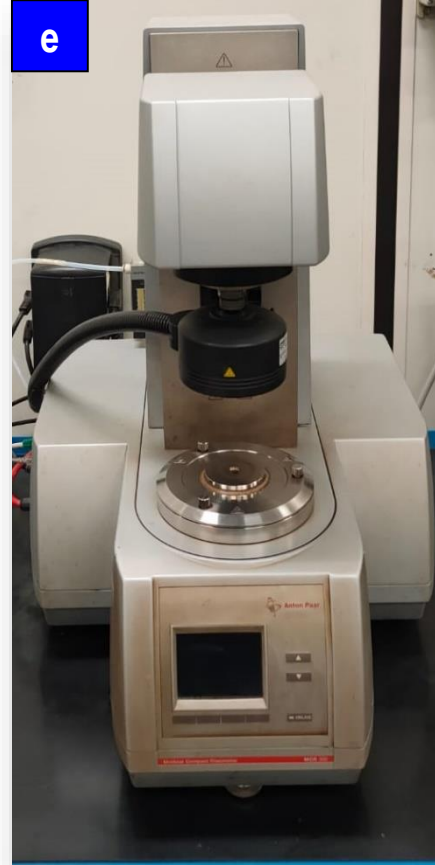


## Methodology

Physicochemical characterization



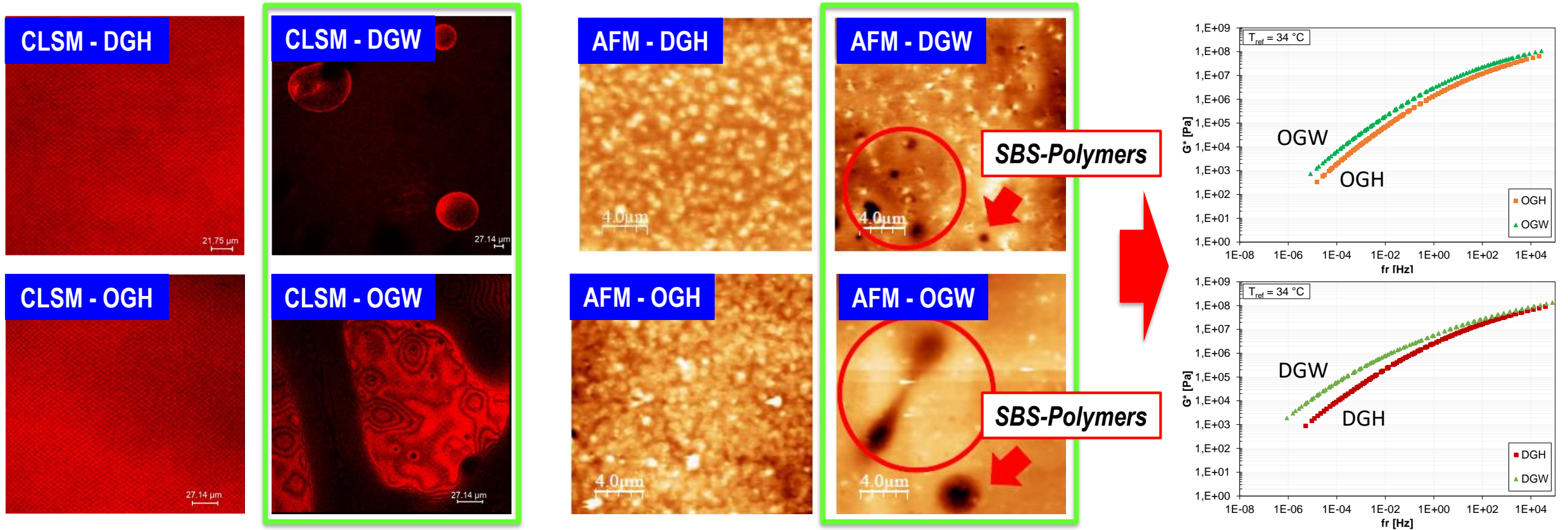
Rheological characterization



WMA recovered binders are less oxidized than the HMA ones

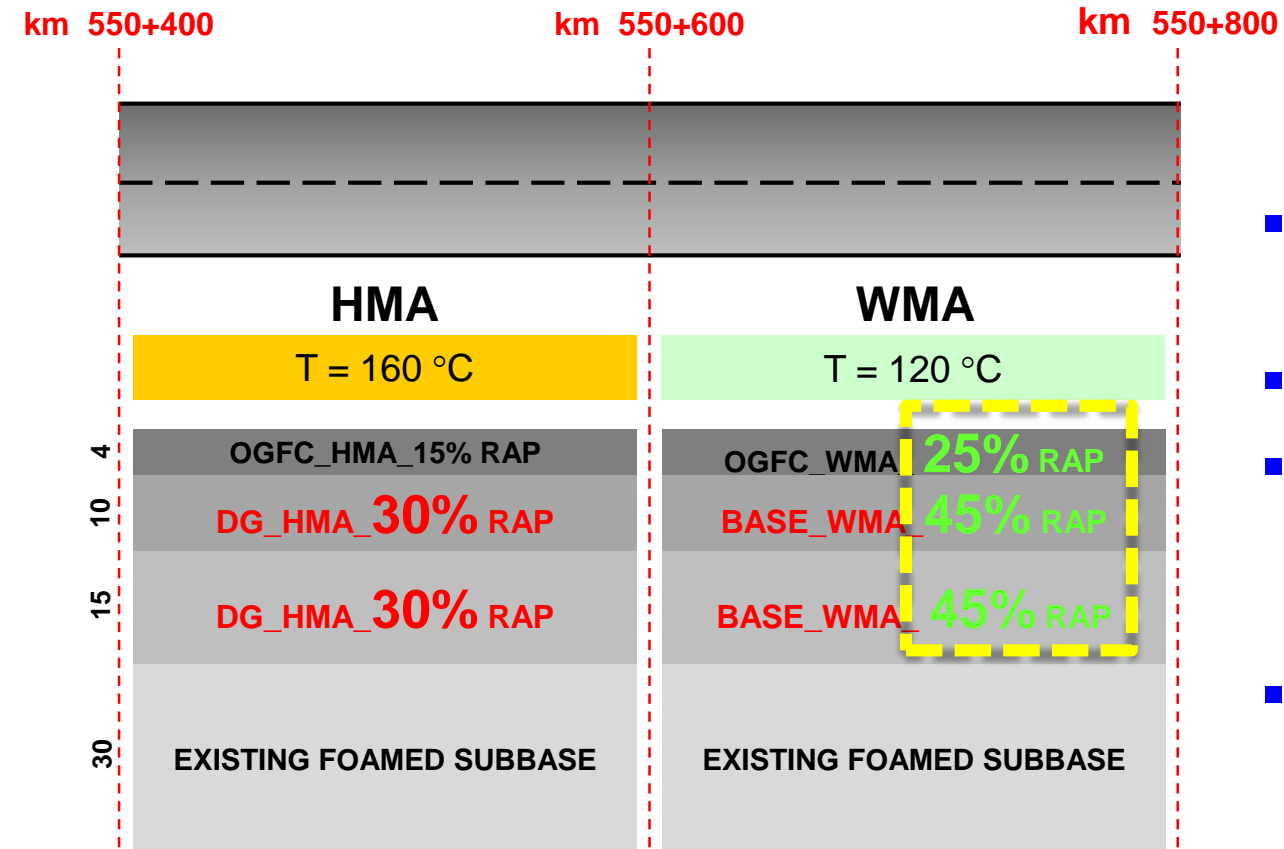
- a. Nuclear Magnetic Resonance (NMR)
- b. Differential Scanning Calorimetry (DSC)
- c. Confocal Laser Scanning Microscopy (CLSM) & Atomic Force Microscopy (AFM)
- d. Frequency Sweep tests (DSR)

## Confocal Laser Scanning Microscopy & Atomic Force Microscopy



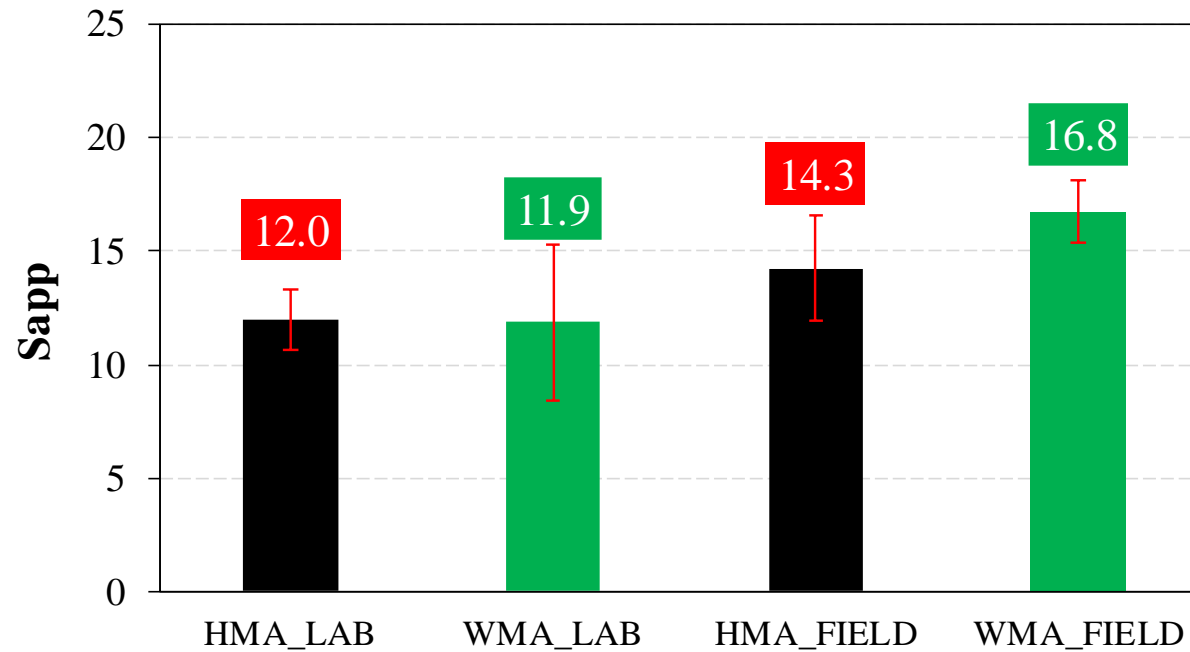
- Same PmB: **SBS-polymers recovered from Warm mixtures are less degraded** (especially DGW).
- Lower WMA T → Lower oxidation but prevent polymer degradation → Higher Stiffness**

## Motorway A1: 2 sections containing PMB



- **Full-depth reconstruction** after milling all existing asphalt layers
- Reference section with **hot recycled** mixtures
- Section with **DG warm recycled** mixtures **containing higher RAP** content, produced with chemical additive C1
- $T_{mix} - T_{comp}$ : HMA 170-160    WMA 130-120

### S-VECD Analysys: HMA vs WMA\_lab and in-situ cores (2022)

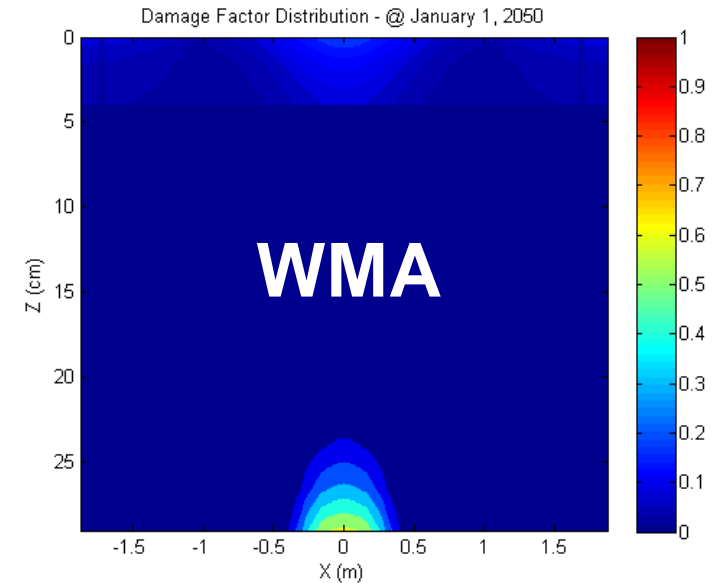
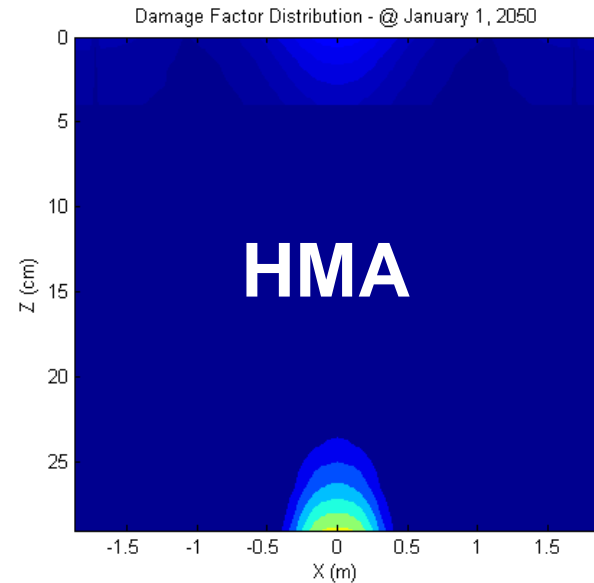


- Sapp values → reflect the observations on the *C-S curves* and  $D^R$  values  
→ **A similar or better fatigue performance can be expected from WMA, slightly higher in field conditions, thanks to the lower RAP oxidation**

### S-VECD Analysys: HMA vs WMA\_lab and in-situ cores (2022)

#### Damage contours

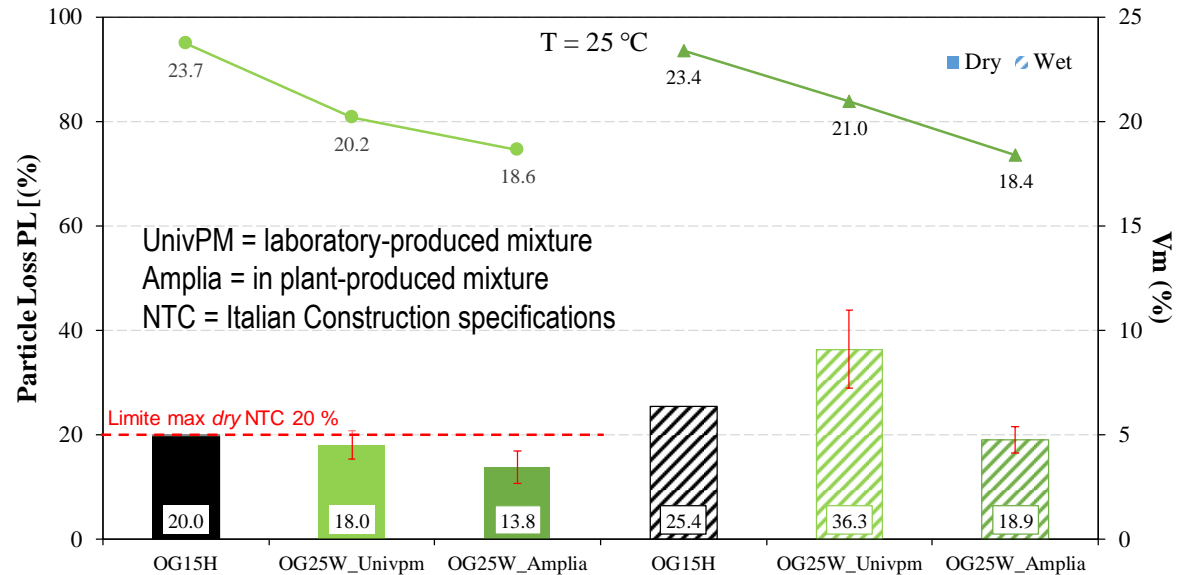
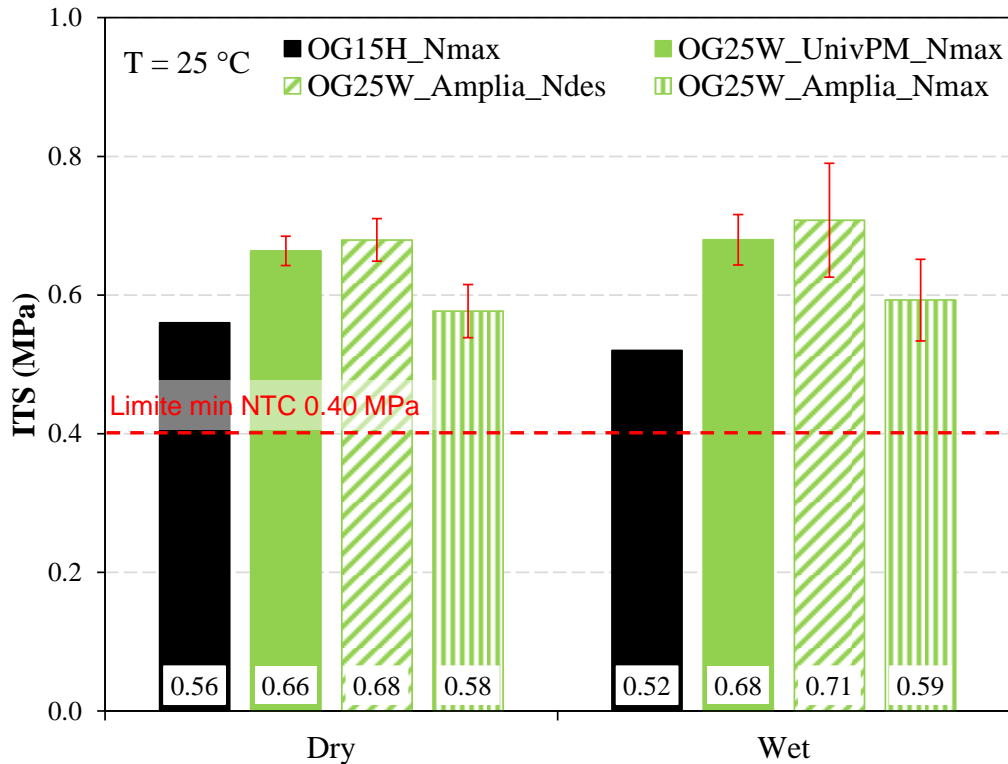
- 32 cm subbase **E = 1200 MPa**  
(conventional unbound subbase)
- Prediction after 30 years
- Fatigue + climatic damage
- 8.5 mln – 120 kN ESALs
- $v = 90$  km/h



- Limited **top-down cracking** due to **thermal effects** in both sections
- **Bottom-up cracking** → about **7 cm** in **HMA** section vs. about **6 cm** in **WMA** section
- **Comparable performance of WMA and HMA mixtures**

## HMA vs WMA\_laboratory-compacted specimens (2022)

### Indirect Tensile & Cantabro Tests



- **ITS: OG\_WMA > OG\_HMA** (both wet and dry conditions)
- Good raveling resistance
- OG\_WMA with 25% RAP satisfies the Construction Specs



## Production at the asphalt plant

1. Drying/heating **aggregate**
2. Heating of the **PmB** ( $T = 170\text{ °C}$ )
3. **Mixing** (WMA: **30 sec** @  $130\text{ °C}$ ; HMA: **26 sec** @  $170\text{ °C}$ )



Asphalt Batch Mix Plant - *Amman Speedy Batch 180* with RAD90 drum

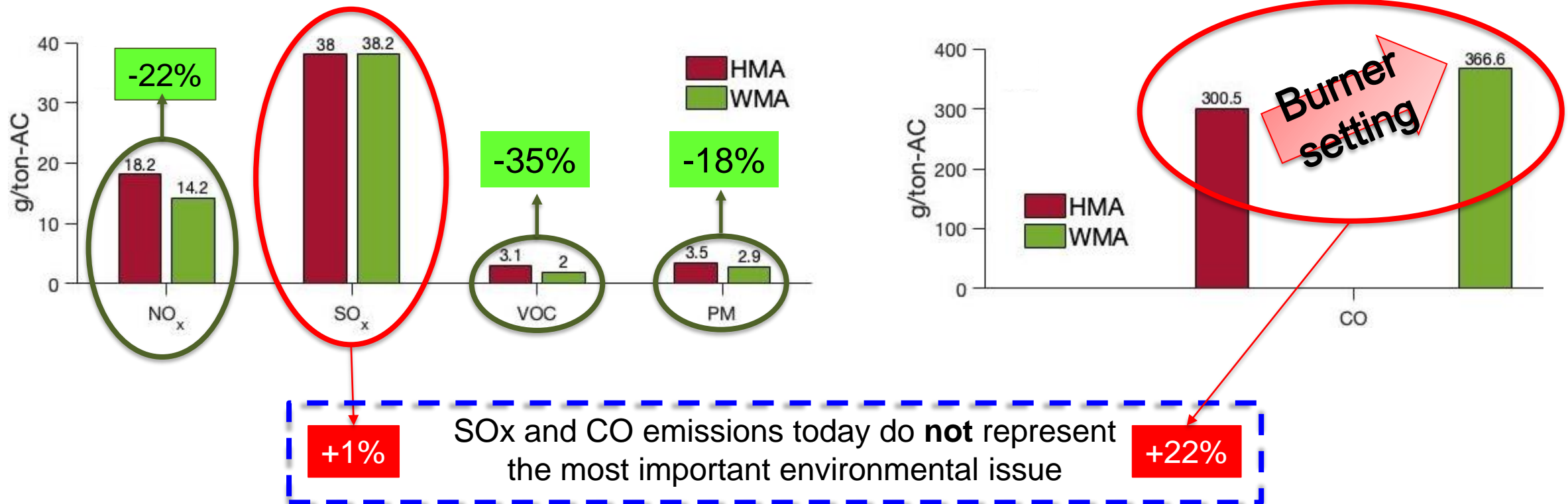
## Calculated Parameters

### Total CO<sub>2</sub> emissions per ton

Production phase	CO <sub>2</sub> emissions [kg/ton]								
	DGAC_base			DGAC_binder			OGAC_wearing		
	WMA	HMA	Δ	WMA	HMA	Δ	WMA	HMA	Δ
Heating aggregates	14.33	16.77	-2.45	14.62	17.22	-2.61	14.34	17.20	-2.86
Heating bitumen	0.79	0.79	—	0.92	0.92	—	1.19	1.19	—
Mixing	0.031	0.027	0.004	0.031	0.027	0.004	0.031	0.027	0.004
<b>Total</b>	15.15	17.59	<b>-2.44</b>	15.57	18.17	<b>-2.60</b>	15.56	18.42	<b>-2.86</b>
			<b>-14%</b>			<b>-14%</b>			<b>-16%</b>

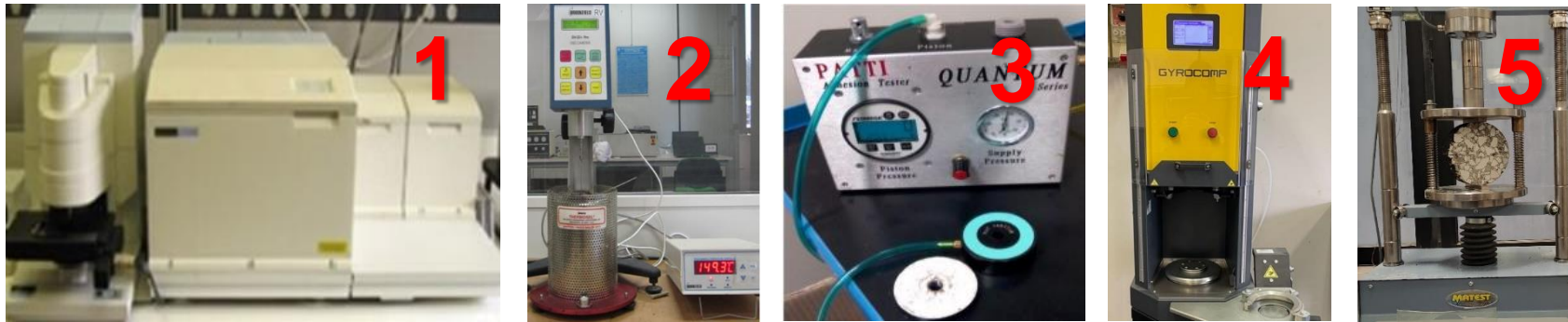
## Measured Parameters

Airborne pollutants emissions **at the stack of the drying drum** during the heating of aggregates

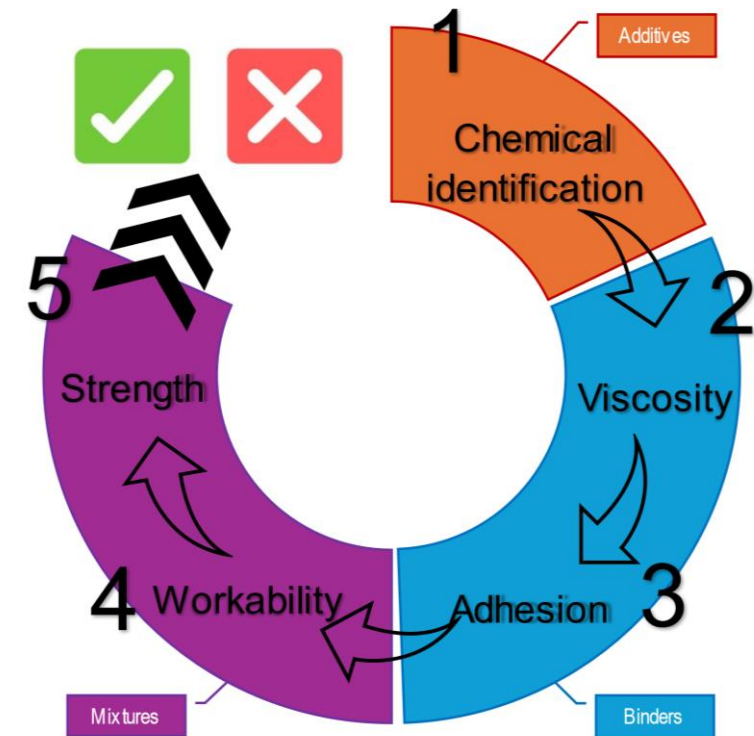


## Objective and testing protocols

Identifying a reliable experimental procedure to evaluate the effectiveness of WMA (chemical) additives

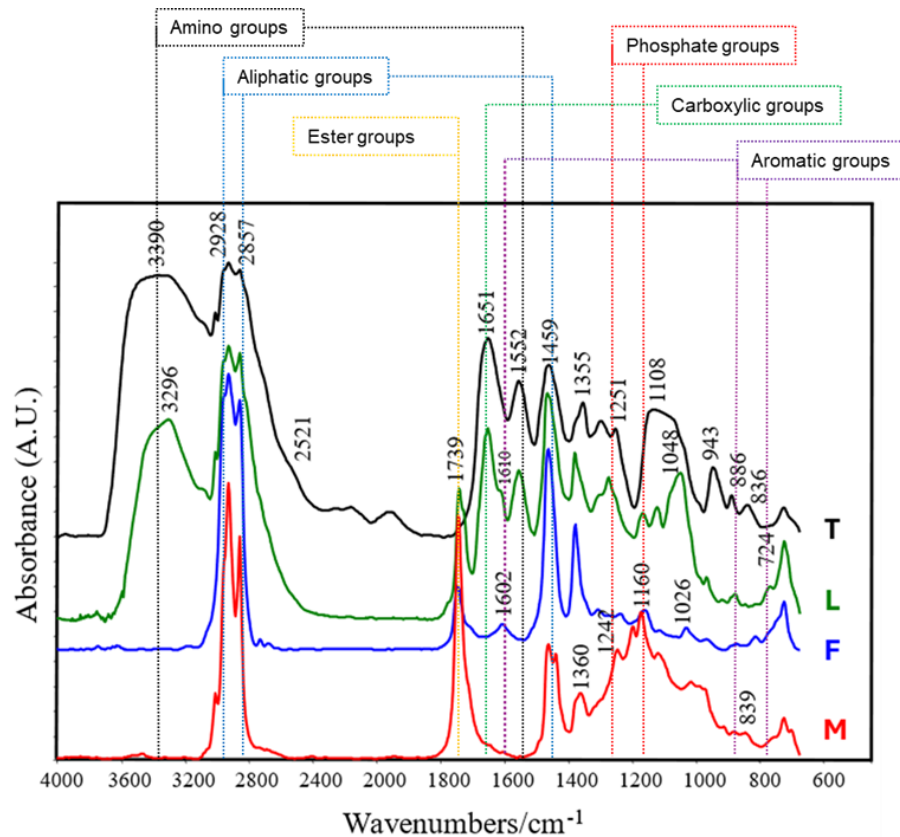


Step	Objective	Method	Parameter	Material	Repetitions
1	Chemical identification	FTIR	Functional groups	Bitumen, Additive	3
2	Viscosity	RV	$\eta$	Bitumen, Bitumen + Additive	2
3	Adhesion	BBS	POTS	Bitumen + Additive, aged Bitumen + Additive	5
4	Workability	SGC	CEI, AV	Hot-mix, Warm-mix + Additive	3
5	Strength	ITT	ITS	Hot-mix, Warm-mix + Additive	3

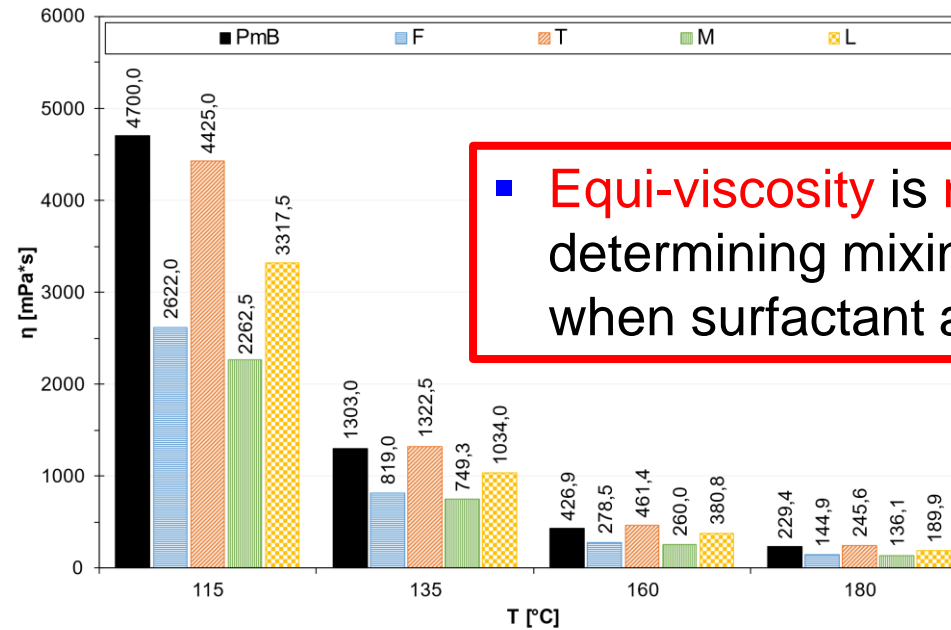


## Step 1 & 2: Chemical identification (FT-IR) & Viscosity (RV)

- **Different Functional Groups can be identified** (amino, aliphatic, phosphate and esters groups) and linked to the adhesion and viscosity of the binder-additive blend.

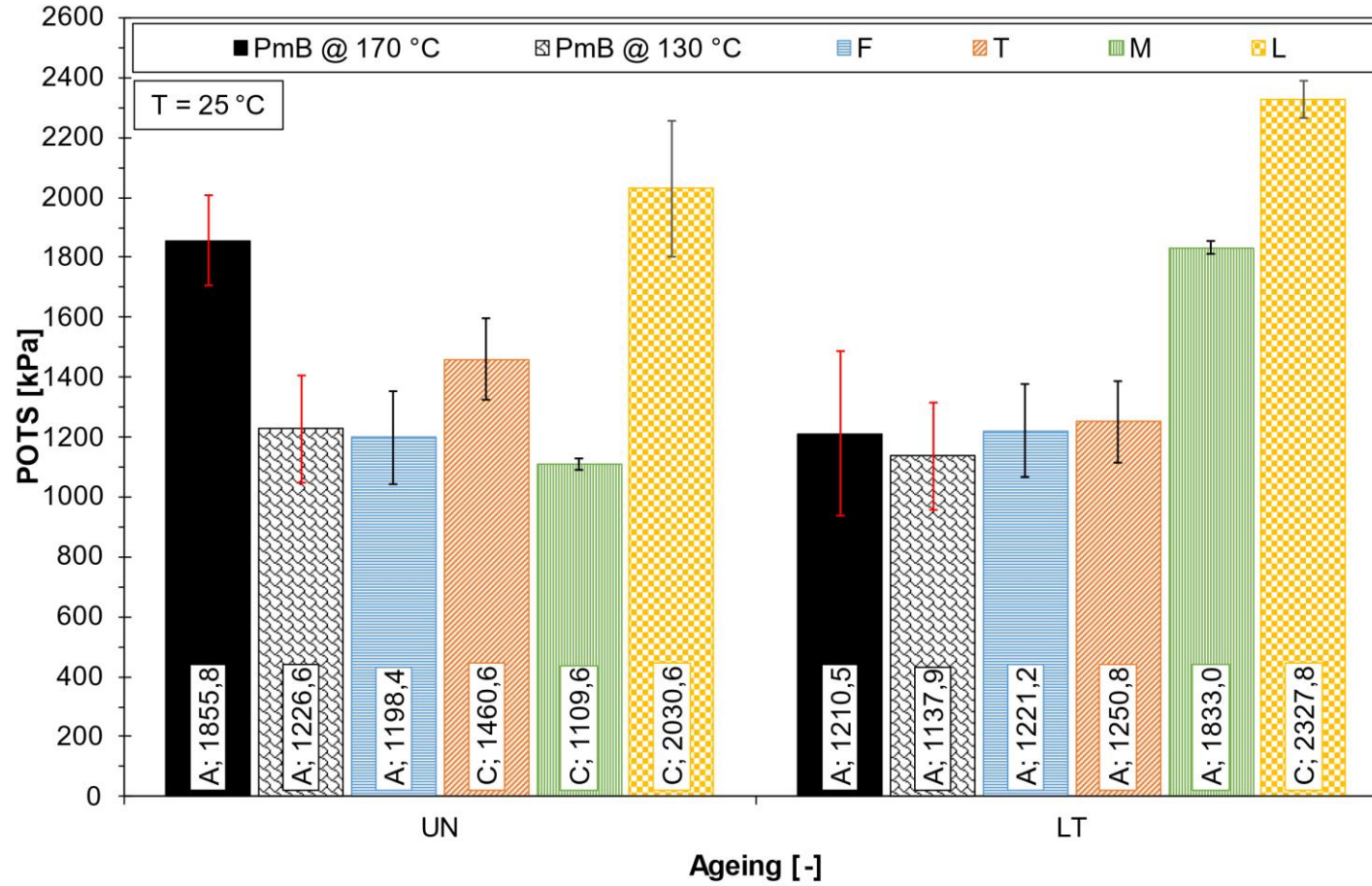


- Additives **F** and **M** act as a **viscosity reducer** whereas additive **T** acts as a **surfactant**



■ **Equi-viscosity is not suitable** for determining mixing and compaction T when surfactant additives are used

## Step 3: Influence on the adhesion properties in wet condition (BBS)



A – Adhesive



C – Cohesive

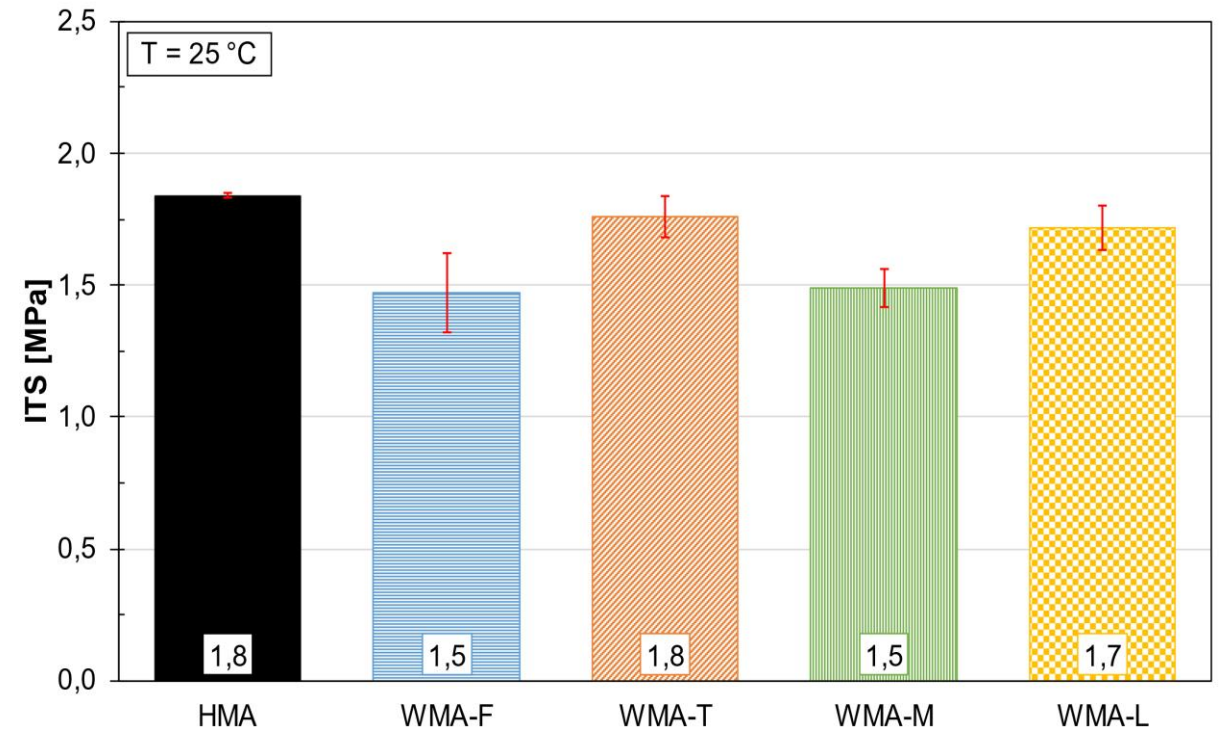
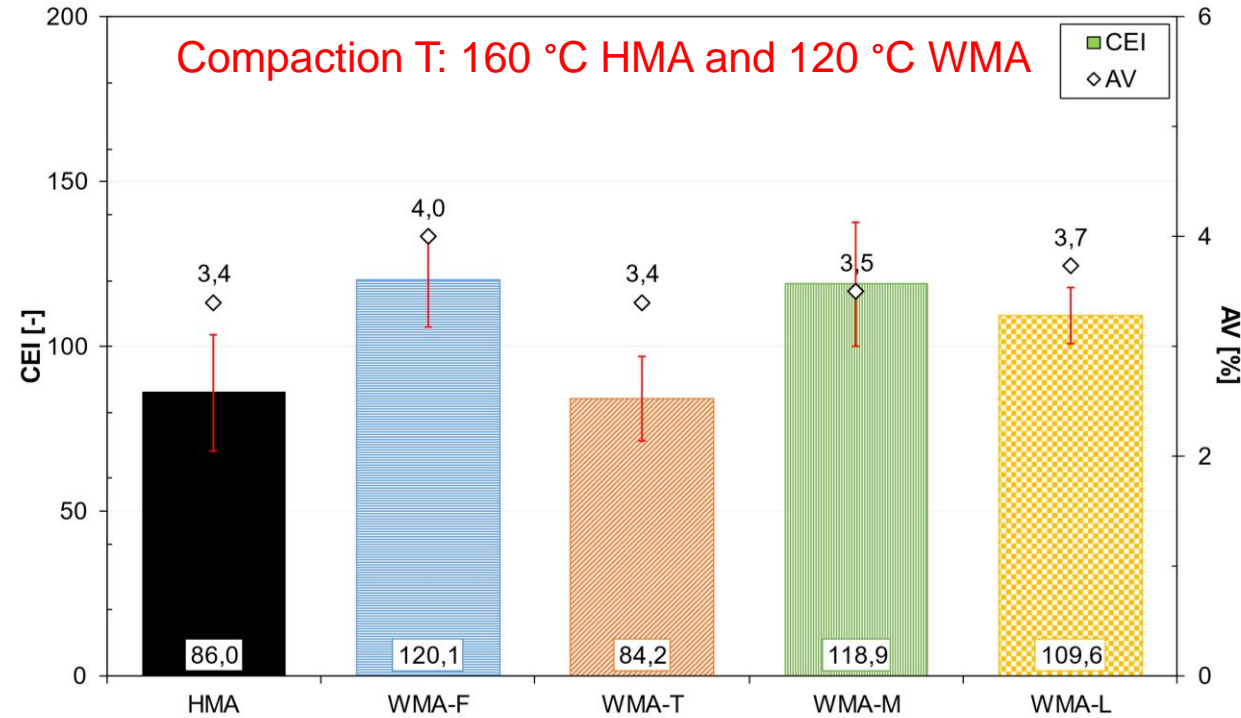


H – Hybrid



- Some additives reduce the adhesion in unaged bitumen (**F**, **M** and partially **T**) due to “warm” test temperature
- Some additives restore adhesion of long-term aged bitumen (**M** and **L**).

## Step 4 & 5: Mixture workability (CEI) and Strength (ITS)



- AV and CEI parameters of warm mixtures are strongly affected by the chemical additive used

- Further validation with advanced mechanical testing (in progress).

## Synthesis of Requirements

- RAP can incorporate a considerable amount of water if exposed to rain up to 5% or higher (up to 8% during extensive precipitation).
- Stockpiling RAP in open sided sheds and on a paved and sloped surface is recommended to allow the water to escape while protecting the RAP from rain;
- Before introducing RAP in the pugmill:  
RAP moisture content  $\leq 4\%$  for HMA whereas  $\leq 3\%$  for WMA (RAP  $\leq 30\%$ )
- Increased minimum bitumen content (lower RAP bitumen reactivation)  
AC base course: total bitumen content  $\geq 4,0\%$  for HMA (RAP  $\leq 30\%$ )  
total bitumen content  $\geq 4,3\%$  for WMA (RAP  $\leq 30\%$ )
- Longer mixing time is recommended to promote RAP-Aggregate heat transfer:  
Typically, 15-20% extended mixing time is suggested
- Lower production and compaction temperatures ( $-40\text{ °C}$ ):  
Production:  $T \leq 170\text{ °C}$  for HMA whereas  $T \leq 130\text{ °C}$  for WMA  
Compaction:  $T \leq 160\text{ °C}$  for HMA whereas  $T \leq 120\text{ °C}$  for WMA





## Some numbers related to ASPI motorway network 2023

### Total asphalt concrete (2023)

- DGAC 524800 ton → **Warm** DGAC 46800 ton (**9%**)
- OGAC 462000 ton → **Warm** OGAC 63150 ton (**14%**)
- **-300 ton CO2 equivalent**
- **≈ -17% fuel consumption**

(source: EPD AMPLIA Magliano Sabina – Asphalt Batch Mix Plant - *Amman Speedy Batch 180* with RAD90 drum)

<i>Mixture</i>	<i>ΔRAP (%)</i>	<i>Production (ton)</i>	<i>ΔRAP (ton)</i>
DGAC	+15%	2700 ton	+410 ton
OGAC	+15%	25850 ton	+3870 ton
	+25%	19300 ton	+4830 ton

**+9110 ton**

## Warm-Recycled Asphalt Mixtures as “Win-Win” Technology

Based on experimental results in terms of:

- Multi-approach analysis of mechanical performance & Interface bonding
- In-situ NDT monitoring of pavement bearing capacity evolution
- Chemical effects on PmBs
- Environmental assessment

**Road Agencies: Higher Performance + Lower Pollution + Circularity**

**Construction Companies: Easy Production & Construction Implementation  
+ Low additional costs (in Italy < 2 Euro/ton<sub>AC-mix</sub>)**



- A. Stimilli, F. Frigio, G. Ferrotti, S. Sciolette, F. Canestrari, “*In-plant production of warm recycled mixtures: a case study*”, International Conference on Transport Infrastructure and Systems TIS2017, pp. 143–151, Rome, 2017.
- A. Stimilli, A. Virgili, F. Canestrari, “*Warm recycling of flexible pavements: effectiveness of WMA additives on SBS modified bitumen and mixture performance*”, Journal of Cleaner Production, vol. 156, pp. 911-922, 2017.
- A. Stimilli, F. Frigio, F. Cardone, F. Canestrari, “*Performance of warm recycled open and dense graded mixtures in field trial sections*”, 10th International Conference on the Bearing Capacity of Roads, Railways and Airfields (BCRRA), 2017.
- F. Frigio, A. Stimilli, A. Virgili, F. Canestrari, “*Performance Assessment of In-Plant-Produced Warm Recycled Mixtures for Open-Graded Wearing Courses*”, Transportation Research Record: Journal of the Transportation Research Board, No. 2633, pp. 16-24, 2017.
- F. Frigio, F. Canestrari, “*Characterization of warm recycled porous asphalt mixtures prepared with different WMA additives*”, European Journal of Environmental and Civil Engineering, Vol. 22, Issue 1, pp. 82-98, 2018.
- L.P. Ingrassia, G. Ferrotti, F. Cardone, F. Canestrari, “*Monitoring the evolution of the structural properties of warm recycled pavements with Falling Weight Deflectometer and laboratory tests*”, Road Materials and Pavement Design, 2021.
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